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#### **DESIGN CERTIFICATE**

Technical basis for structural design methodology contained in designIT for houses - New Zealand.

designIT for houses, New Zealand has been developed by experienced timber engineers to assist designers in selecting appropriate sizes of structural laminated veneer lumber products manufactured by Carter Holt Harvey LVL Limited (including hySPAN, hy90, hyONE and hyJOIST) and other generic stress grades of timber, to be used as structural elements for the construction of buildings that fall within the scope of NZS 3604.

The design methodology used for the software complies with the loading and general design requirements contained within AS/NZS 1170 and with timber structural design in accordance with NZS 3603:1993 including Amendment 4 (Verification method B1/VM1, 6.1).

designIT relies on the accurate input of span and loading information by the user. Where accurate inputs are submitted the product and/or stress grade and the size given will comply with the structural requirements of the New Zealand Building Code (NZBC), provided the installation is in accordance with the installation requirements provided by designIT and/or in product literature and/or NZS 3604, or specific engineering design, as appropriate.

Futurebuild LVL and SG8 components, when used and treated to the required treatment levels prescribed in NZS 3602 and NZS 3604, as modified by Acceptable Solution B2/AS1, will comply with the requirements of the NZBC (Acceptable Solution B2/AS1, 3.2).

#### References:

- 1. NZS 3603:1993 Timber Structures Standard.
- 2. NZS 3604:2011 Timber-framed buildings.
- 3. AS/NZS 1170:2002 Structural design actions, Parts 0 and 1.
- 4. AS/NZS 1170:2011 Structural design actions, Part 2: Wind actions.
- 5. AS/NZS 1170:2003 Structural design actions, Part 3: Snow and ice actions.
- 6. AS 1720.1:2010 Timber structures. Part 1: Design methods.
- 7. AS 1720.3:2016 Timber structures. Part 3: Design criteria for timber-framed residential buildings.

This Design Certificate, and any associated warranty/certification, is void where there has been substitution of alternate products not detailed within the Member Specification.

Version date: 5 October 2021

For further information or advice contact: Carter Holt Harvey LVL Limited, 173 Captain Springs Road, Onehunga. Auckland Telephone: 0800 808 131 Email: designit@futurebuild.co.nz Web: https://futurebuild.co.nz/

#### Specifier details:

Specifier:	Reshma Ravindranath
Business name:	Carters Manufacturing Wellington
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New House Cromwell 2
358544C1
64 Waipounamu Drive (Cromwell2) Kelson Lower Hutt
D.M.A.C Homes Ltd   T/A Friday Homes
High
Design snow zone: N1, Altitude: 100 m (sub-alpine), Ground snow load, Sg <sup>1,2</sup> = 0.0 kPa BUIL DING CONS

designIT does not include any allowance for the effects of drifting and sliding of snow.

. Snow loads are applied to roofed over structures only, the design of exposed floors/decks are not covered by designIT.

#### MEMBER DESIGN DETAILS

#### Member 1

1) Member code and description

EJ01 - Lintels - In single or upper storey load bearing walls

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2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	1.4 m
Roof load width 'RLW'	1.8 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member energiaction	
5) Member specification	
Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	4.7 mm	0.4 mm (long term)	11.0
Live load - Ψ <sub>s</sub> Q	5.6 mm	0.1 mm	45.5
Wind load - Ws	7.0 mm	0.4 mm	18.9

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-1.1
1.2G + 1.5Q	0.80	-1.6
1.2G + Wu + ΨcQ	1.00	-2.3
0.9G + Wu	1.00	1.4

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 2

1) Member code and description

EJ02 - Lintels - Supporting girder truss / truncated girder truss



#### 2) Date prepared

3) Serviceability criteria

- 4) Design inputs
  - Roof type Roof pitch Truss spacing

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AS 1720.1: 2010 and AS 1720.3: 2016

Light roof and ceiling - 40 kg/m<sup>2</sup> 25.0 ° 900 mm

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	Eaves overhang	600 mm
	TGT span	8300 mm
	TGT setback	2327 mm
	TGT offset	837 mm
	Lintel span	2020 mm
	Nominal wall thickness	90 mm
5) I	Member specification	
	Size, stress grade/product	Use 200 x 90 hy90
	Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_L Q^*$	6.7 mm	2.4 mm (long term)	2.8
Live load - Ψ <sub>s</sub> Q	8.1 mm	0.7 mm	11.2
Wind load - Ws	10.1 mm	2.2 mm	4.6

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-4.1
1.2G + 1.5Q	0.80	-6.3
$1.2G + W_u + \Psi_c Q$	1.00	-8.9
0.9G + W <sub>u</sub>	1.00	5.7

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 3

1) Member code and description	EJ03 - Lintels - In single or upper storey load bearing walls	
2) Date prepared	01 December 2021	
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016	
4) Design inputs		
Span	0.9 m	
Roof load width 'RLW'	5.1 m	[
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>	BUILDING CONSENT
Nominal wall thickness	90 mm	
5) Member specification		GRANTED
Size, stress grade/product	Use 2/90 x 45 SG8	28/03/2022
Material type	Dry softwood, machine stress graded and verified (NZS 3622)	
Assumed design density	< 480 kg/m²	HUTT CITY COUNCIL
6) Sonvigoability		

6) Serviceability



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Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	3.0 mm	1.7 mm (long term)	1.8
Live load - Ψ <sub>s</sub> Q	3.6 mm	0.5 mm	7.0
Wind load - Ws	4.5 mm	1.5 mm	2.9

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-2.8
1.2G + 1.5Q	0.80	-4.5
$1.2G + W_u + \Psi_c Q$	1.00	-6.3
0.9G + W <sub>u</sub>	1.00	4.2

#### 8) Installation requirements

- Provide at least 30 mm bearing at end supports
- Vertical lamination required refer AS 1684

#### Member 4

1) Member code and description	EJ04 - Lintels - In single or upper storey load bearing walls
2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	0.9 m
Roof load width 'RLW'	5.1 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_L Q^*$	3.0 mm	1.7 mm (long term)	BUILDING CONSENT
Live load - Ψ <sub>s</sub> Q	3.6 mm	0.5 mm	BUILDING CONSENT
Wind load - W <sub>s</sub>	4.5 mm	1.5 mm	2.9 GRANTED
See 'Notes for interpretation of servic 7) Reactions	eability data' at the end of this report	Limit States Design Rea	28/03/2022
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>	
1.35G	0.60	-2.8	
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1.2G + 1.5Q	0.80	-4.5
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-6.3
0.9G + Wu	1.00	4.2

#### 8) Installation requirements

Provide at least 30 mm bearing at end supports

• Vertical lamination required - refer AS 1684

#### Member 5

1) Member code and description	EJ05 - Lintels - In single or upper storey load bearing walls	
2) Date prepared	01 December 2021	
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016	
4) Design inputs		
Span	2.1 m	
Roof load width 'RLW'	5.1 m	
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>	
Nominal wall thickness	90 mm	
5) Member specification		
Size, stress grade/product	Use 150 x 90 hy90	
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357	

#### 6) Serviceability

Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
7.0 mm	3.9 mm (long term)	1.8
8.4 mm	1.2 mm	7.1
10.5 mm	3.5 mm	3.0
	7.0 mm 8.4 mm	7.0 mm         3.9 mm (long term)           8.4 mm         1.2 mm

\*Critical serviceability load case See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-4.1
1.2G + 1.5Q	0.80	-6.5
1.2G + Wu + ΨcQ	1.00	-9.1
0.9G + Wu	1.00	5.9

#### 8) Installation requirements

Provide at least 30 mm bearing at end supports

#### Member 6

1) Member code and description

EJ06 - Lintels - Supporting girder truss / truncated girder truss

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#### 2) Date prepared

3) Serviceability criteria

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AS 1720.1: 2010 and AS 1720.3: 2016

#### 4) Design inputs

-	• •	
	Roof type	Light roof and ceiling - 40 kg/m <sup>2</sup>
	Roof pitch	25.0 °
	Truss spacing	900 mm
	Eaves overhang	600 mm
	TGT span	4320 mm
	TGT setback	1405 mm
	TGT offset	465 mm
	Lintel span	2020 mm
	Nominal wall thickness	90 mm
)	Member specification	

#### 5)

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_L Q^*$	6.7 mm	2.8 mm (long term)	2.4
Live load - Ψ <sub>s</sub> Q	8.1 mm	0.8 mm	9.9
Wind load - Ws	10.1 mm	2.5 mm	4.1
*Critical serviceability load case			

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-2.0
1.2G + 1.5Q	0.80	-3.1
1.2G + Wu + ΨcQ	1.00	-4.3
0.9G + Wu	1.00	2.7

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 7

1) Member code and description

EJ07 - Lintels - In single or upper storey load bearing walls

2) Date prepared

01 December 2021

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3) Serviceability c	riteria	AS 1720.1: 2010 and AS 1720.3: 2016	
4) Design inputs			
Span		3.1 m	
Roof load width	ר 'RLW'	1.3 m	
Roof type and	mass	Light roof and ceiling - 40 kg/m <sup>2</sup>	
Nominal wall th	nickness	90 mm	
5) Member specifi	ication		
Size, stress gra	ade/product	Use 200 x 90 hy90	
Material type		Structural Laminated Veneer Lumber to AS/NZS 4357	

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	10.0 mm	2.7 mm (long term)	3.7
Live load - Ψ <sub>s</sub> Q	12.4 mm	0.7 mm	17.6
Wind load - Ws	15.5 mm	2.1 mm	7.3
*Critical serviceability load case			

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-1.5
1.2G + 1.5Q	0.80	-2.3
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-3.1
0.9G + Wu	1.00	1.8

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 8

1) Member code and description	EJ08 - Lintels - In single or upper storey load bearing walls
2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	0.9 m
Roof load width 'RLW'	1.0 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

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#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	3.0 mm	0.3 mm (long term)	8.9
Live load - Ψ <sub>s</sub> Q	3.6 mm	0.1 mm	35.9
Wind load - Ws	4.5 mm	0.3 mm	14.9
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\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-0.6
1.2G + 1.5Q	0.80	-0.9
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-1.3
0.9G + Wu	1.00	0.8

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

• Vertical lamination required - refer AS 1684

#### Member 9

1) Member code and description	EJ09 - Lintels - In single or upper storey load bearing walls
2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	2.3 m
Roof load width 'RLW'	1.8 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigi	dity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	7.7 mm	2.0 mm (long term)		IL DING CONSENT
Live load - $\Psi_s Q$	9.2 mm	0.5 mm	BU	16.9
Wind load - Ws	11.5 mm	1.6 mm		7.0
*Critical serviceability load case See 'Notes for interpretation of serviceability data' at the end of this report 7) Reactions			22	28/03/2022
		Limit States Design Rea	ction <sup>2</sup> , <sup>3</sup> HU	TT CITY COUNCIL
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>		
1.35G	0.60	-1.6		
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1.2G + 1.5Q	0.80	-2.5
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-3.5
0.9G + Wu	1.00	2.1

#### 8) Installation requirements

Provide at least 30 mm bearing at end supports

#### Member 10

1) Member code and description	EJ10 - Lintels - In single or upper storey load bearing walls	
2) Date prepared	01 December 2021	
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016	
4) Design inputs		
Span	0.7 m	
Roof load width 'RLW'	5.1 m	
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>	
Nominal wall thickness	90 mm	
5) Member specification		
Size, stress grade/product	Use 2/90 x 45 SG8	
Material type	Dry softwood, machine stress graded and verified (NZS 3622)	
Assumed design density	< 480 kg/m²	

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	2.3 mm	0.8 mm (long term)	3.0
Live load - ΨsQ	2.8 mm	0.2 mm	11.6
Wind load - Ws	3.5 mm	0.7 mm	4.8

\*Critical serviceability load case See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-2.7
1.2G + 1.5Q	0.80	-4.2
1.2G + Wu + ΨcQ	1.00	-6.0
0.9G + Wu	1.00	3.9

#### 8) Installation requirements

- Provide at least 30 mm bearing at end supports
- · Vertical lamination required refer AS 1684

#### Member 11

1) Member code and description

EJ11 - Lintels - In single or upper storey load bearing walls

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2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	2.1 m
Roof load width 'RLW'	5.1 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	7.0 mm	3.9 mm (long term)	1.8
Live load - ΨsQ	8.4 mm	1.2 mm	7.1
Wind load - Ws	10.5 mm	3.5 mm	3.0

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-4.1
1.2G + 1.5Q	0.80	-6.5
1.2G + Wu + ΨcQ	1.00	-9.1
0.9G + W <sub>u</sub>	1.00	5.9

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 12

1) Member code and description	EJ12 - Lintels - In single or upper storey load bearing walls	
2) Date prepared	01 December 2021	
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016	
4) Design inputs		
Span	0.7 m	B
Roof load width 'RLW'	5.1 m	
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>	
Nominal wall thickness	90 mm	
5) Member specification		
Size, stress grade/product	Use 2/90 x 45 SG8	H
Material type	Dry softwood, machine stress graded and verified (NZS 3622)	
Assumed design density	< 480 kg/m²	

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#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	2.3 mm	0.8 mm (long term)	3.0
Live load - Ψ <sub>s</sub> Q	2.8 mm	0.2 mm	11.6
Wind load - Ws	3.5 mm	0.7 mm	4.8

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-2.7
1.2G + 1.5Q	0.80	-4.2
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-6.0
0.9G + Wu	1.00	3.9

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

• Vertical lamination required - refer AS 1684

#### Member 13

1) Member code and description	EJ13 - Lintels - In single or upper storey load bearing walls
2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	2.1 m
Roof load width 'RLW'	2.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size. stress grade/product	Use 150 x 90 hy90

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Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	BiddleDiNG CONSENT
Long term load - G + $\Psi_LQ^*$	7.0 mm	1.9 mm (long term)	3.8
Live load - Ψ <sub>s</sub> Q	8.4 mm	0.5 mm	<sup>15.</sup> <b>GRANTED</b>
Wind load - Ws	10.5 mm	1.6 mm	<sup>6.6</sup> 28/03/2022
*Critical serviceability load case See 'Notes for interpretation of service 7) Reactions	eability data' at the end of this report		
		Limit States Design Rea	action <sup>2,3</sup>
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Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-1.9
1.2G + 1.5Q	0.80	-3.0
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-4.2
0.9G + W <sub>u</sub>	1.00	2.6

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 14

1) Member code and description	EJ14 - Lintels - In single or upper storey load bearing walls
2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	2.1 m
Roof load width 'RLW'	1.4 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	7.0 mm	1.2 mm (long term)	5.9
Live load - Ψ <sub>s</sub> Q	8.4 mm	0.3 mm	26.0
Wind load - Ws	10.5 mm	1.0 mm	10.8

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction	n <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>	
1.35G	0.60	-1.2	
1.2G + 1.5Q	0.80	-1.8	
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-2.6	
0.9G + Wu	1.00	1.6	BUILDING CONSENT
<ul><li>8) Installation requirements</li><li>Provide at least 30 mm bearing at end</li></ul>	supports		GRANTED
· ·	supports		GRANTED 28/03/2022
Provide at least 30 mm bearing at end	supports		
, .		upper storey load bearing walls	28/03/2022





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2) Date prepared	01 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	2.5 m
Roof load width 'RLW'	1.8 m
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	8.3 mm	2.8 mm (long term)	2.9
Live load - ΨsQ	10.0 mm	0.8 mm	12.8
Wind load - Ws	12.5 mm	2.4 mm	5.3

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-1.7
1.2G + 1.5Q	0.80	-2.6
1.2G + Wu + ΨcQ	1.00	-3.6
0.9G + Wu	1.00	2.2

#### 8) Installation requirements

• Provide at least 30 mm bearing at end supports

#### Member 16

1) Member code and description	Entry Beam - Lintels - In single or upper storey load bearing walls	
2) Date prepared	01 December 2021	
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016	
4) Design inputs		BU
Span	2.0 m	DU
Roof load width 'RLW'	1.8 m	
Roof type and mass	Light roof and ceiling - 40 kg/m <sup>2</sup>	
Nominal wall thickness	90 mm	
5) Member specification		
Size, stress grade/product	Use 150 x 90 hy90	HU
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357	

#### **BUILDING CONSENT**

GRANTED 28/03/2022

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#### 6) Serviceability

Load case	Limit <sup>3</sup> on average deflection <sup>2</sup>	Estimated average deflection <sup>2</sup>	Rigidity ratio <sup>4</sup>
Long term load - G + $\Psi_LQ^*$	6.7 mm	1.3 mm (long term)	5.2
Live load - $\Psi_s Q$	8.0 mm	0.4 mm	22.3
Wind load - Ws	10.0 mm	1.1 mm	9.2

\*Critical serviceability load case

See 'Notes for interpretation of serviceability data' at the end of this report

#### 7) Reactions

		Limit States Design Reaction <sup>2,3</sup>
Load case	k1 <sup>1</sup>	End kN <sup>4</sup>
1.35G	0.60	-1.5
1.2G + 1.5Q	0.80	-2.3
1.2G + W <sub>u</sub> + Ψ <sub>c</sub> Q	1.00	-3.2
0.9G + Wu	1.00	2.0

#### 8) Installation requirements

· Provide at least 30 mm bearing at end supports

#### Notes for interpretation of serviceability data

- 1. 'average deflection' is an engineering concept based upon a notional estimated load, notional member rigidity and, in some cases, an approximate model of material response to environmental conditions. These parameters are, 'standardised' in AS 1170 and AS 1720.
- 2. Deflection is the flexural response to load 'out-of-level' measurements of installations are not necessarily deflections and can incorporate 'initial out-of-straightness', whether intended or not. Furthermore, loads can be higher/lower than the notional estimate and in any comparison with measured levels, material variability needs to also be considered. AS 1720 gives the following basis for estimation of upper bound deflections for various materials. No 1 Framing – visually graded to NZS 3631
  - SG grades mechanically graded to AS/NZS 1748Average + 43%GL grades for glulam to AS 1328Average + 33%LVL to AS/NZS 4357 (includes hySPAN and hyJOIST)Average + 18%

As can be seen, comparison of the 'average deflection' for different materials, even if calculated on the same basis, does not give the whole picture!

3. The limits referred are those specified in AS 1720.3 for the stated load case.

4. 'Rigidity ratio' expresses the rigidity of the specified beam relative to the rigidity of a notional beam just meeting the serviceability requirements detailed.

#### Notes for interpretation of reaction data

Duration of load factor 'k1' for strength as per NZS 3603:1993

Negative (-) reactions relate to the 'gravity' or 'downwards' force on the support

- Positive reactions relate to the 'upwards' forces or 'tie-down' requirement on the support
- . End reaction includes allowance for overhang/cantilever where one has been designed



## **Demand Calculation Sheet**

Job Details		
Name:	Cromwell (Cromwell 2	
Street and Number:		, 64, 67, 69 & 73 Kelson Heights
Lot and DP Number:		, 64, 67, 69 & 73 DP TBC
City/Town/District:	Lower Hutt	
Designer:	E Horner	
Company:	Prime Designs	
Date:	Tuesday, 11 January	2022
Building Specification		
Number of Storeys	1	
Floor Loading	2 kPa	
Foundation Type	Slab	
	Single	<b>BUILDING CONSENT</b>
Cladding Weight	Light	
Roof Weight	Light	GRANTED
Room in Roof Space	No	28/03/2022
Roof Pitch (degrees)	25	
Roof Height above Eaves (m)	2.4	HUTT CITY COUNCIL
Building Height to Apex (m)	4.9	
Ground to Lower Floor (m)	0.2	
		HUTTCITY
Average Stud Height (m)	2.4	
Building Length (m)	15.9	L
Building Width (m)	8.3	
Building Plan Area (m <sup>2</sup> )	100	

#### **Building Location**

Wind Zone = High	REVISED 23/03/2022 HUTT CITY COUNCIL	Earthquake Zone 3Soil TypeD & E (Deep to Very Soft)Annual Prob. of Exceedance:1 in 500 ( Default)
	HUTTCITY	

#### **Bracing Units required for Wind**

Single Level	Along 435	Across 817

#### Bracing Units required for Earthquake

Along & Across Single Level 531

EQ

Demand

Wind

## Single Level Along Resistance Sheet

Job Name: Cromwell (Cromwell 2)

									435	531
									Achi	eved
Line	Element	Length	Angle	Stud Ht.	Туре	Supplier	Wind	EQ	1190	1083
		(m)	(degrees)	(m)			(BUs)	(BUs)	273%	204%
	1	0.45		2.4	GS1-N	GIB®	24	26		
	2	0.50		2.4	GS1-N	GIB®	28	29		
а	3	0.40		2.4	GS1-N	GIB®	21	23		
	4	3.55		2.4	GS1-N	GIB®	245	213		
				Externa	I Length =	15.7			318 OK	291 OK
	1	3.69		2.4	GS1-N	GIB®	255	221		
h	2	2.00		2.4	GS1-N	GIB®	138	120		
b	3	3.55		2.4	GS2-NOM	GIB®	178	178		-
				Externa	l Lenath =	5.5			570 OK	519 OK
	1	1.28		2.4	GS1-N	GIB®	88	77		
	2	0.69		2.4	GS1-N	GIB®	41	41		
с	3	1.85		2.4	GS1-N	GIB®	128	111		
	4	0.75		2.4	GS1-N	GIB®	45	44		
				Externa	l Length =	15.9			302 OK	272 OK

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GIB EzyBrace® Version 12/18a



EQ

Demand

Wind

## Single Level Across Resistance Sheet

Job Name: Cromwell (Cromwell 2)

									Achi	eved
Line	Element	Length	Angle	Stud Ht.	Туре	Supplier	Wind	EQ	1011	953
		(m)	(degrees)	(m)			(BUs)	(BUs)	124%	179%
	1	0.40		2.4	BL1-H	GIB®	36	40		
	2	0.40		2.4	BL1-H	GIB®	36	40		
m	3	0.40		2.4	BL1-H	GIB®	36	40		
	4	0.40		2.4	BL1-H	GIB®	36	40		
				Externa	al Length =	8.3			142 OK	160 Oł
	1	1.18		2.4	GS1-N	GIB®	81	71		
n	2	0.59		2.4	GS2-NOM	GIB®	30	30		-
									110 OK	100 Oł
	1	1.90		2.4	GS1-N	GIB®	131	114		
0	2	3.12		2.4	GS1-N	GIB®	215	187		
					-				346 OK	301 Oł
	1	1.90		2.4	GS1-N	GIB®	131	114		
р	2	1.79		2.4	GS1-N	GIB®	124	107		
					-				255 OK	221 Oł
	1	0.40		2.4	GS1-N	GIB®	21	23		
	2	0.40		2.4	GS1-N	GIB®	21	23		
q	3	0.50		2.4	GS1-N	GIB®	28	29		
ч	4	0.47		2.4	BL1-H	GIB®	43	47		
	5	0.47		2.4	BL1-H	GIB®	43	47		

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## **Custom Wall Elements**

Supplier	System	Min. Length	Wind	EQ
		m	BUs/m	BUs/m
CHH	EP1 0.4	0.4	80	95
СНН	EP1 0.6	0.6	95	105
СНН	EP1 1.2	1.2	120	135

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#### **BUILDING CONSENT**

GIB

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## **BUILDABLE CONSENT** LAYOUT

For valley/saddle truss fixing unless stated otherwise use a pair of wire dogs at 900mm centres for up to and including a very high wind zone. Or a pair of CT200's at 900mm centres for extra high wind zone. This fixing is to meet the minimum requirements as per NZS3604.







BA

R2 J5A

J5

J5B

J5

J5A

R2

## Your Building Partner

CARTERS Wellington (04) 56702	Manufacturing 87
JOB No	358544C1
Client: DMA	C Homes Ltd T/A F

Friday Homes /IAC Homes Ltd Job: New House- Cromwell 2 Site: 64 Waipounamu Drive (Cromwell 2)

Kelson

Lower Hutt

Pitch: 25.0deg
Roof Type: Longrun
Overhang: 450mm
Wind Area: High
Roof Snow: 0.000kPa
Ceiling Restraint Centres:600mm

Trusses and rafters at 900mm max centres unless stated otherwise. This layout is to be read in conjunction with the Architectural plans.

1 Dec,2021

**DRAWN** Reshma FIXINGS

- A = 47x90 Joist Hanger B = 47x120 Joist Hanger C = CT200 (pair) D = 47x190 Joist Hanger E = 95x165 Joist Hanger F = SH-140 Split Hanger G = SH-180 Split Hanger H = SH-220 Split Hanger J = 2x6kN Strap (12kN Total) K = 6kN Strap L = Multigrip (single) M = Multigrips (pair)
- N = Nailon Plate (240x110x1)
- P = 16kN Pack
- Q = 9kN Pack
- S = CPC 40 Single Cleat T = CPC 40 Short (pair)
- U = CPC 80 Single Cleat
- V = 16kN Uplift
- W= 24kN Uplift
- X = 25kN Uplift
- Y = 35kN Uplift Z = 45kN Uplift

Unless otherwise indicated, all specified truss fixings are to use L/Lok product nail fasteners (as per the MiTek On-site Guide) when the choice of using screws or nails is optional.

All truss to frame fixings require 2 additional 2/90x3.15dia skew nails

All truss fixings not indicated as above must have 2 wire dogs for cross joints and 2/90x3.15dia nails for butt joins.

Fixings shown are for fixing trusses to the top plate. Any other point load uplift fixings down through the framing stud to top plate, stud to bottom plate, bottom plate to floor remain the responsibility of the architect / draughtsman.

**Truss Layout** 

## **BUILDABLE CONSENT** LAYOUT





Lintel fixing specification remains the responsibility of the architect / draughtsperson





## Your **Building Partner**



MiTek 20/20 Engineering 4.7.346.0

MiTek New Zealand Limited

Correspondence from : AUCKLAND 40 Neales Road, East Tamaki 2013 PO Box 58-014, Botany 2163 Phone: 09 274 7109 Fax: 09 274 7100 CHRISTCHURCH 14 Pilkington Way, Wigram 8042 PO Box 8387, Riccarton 8440 Phone: 03 348 8691 Fax: 03 348 0314

www.miteknz.co.nz Printed: 15:49:58 07 Dec 2021

PRODUCER STATEMENT for MiTek 20/20<sup>®</sup> TRUSS DESIGN - Version 4.7

ISSUED BY: MiTek New Zealand Limited

TO: <u>CARTERS</u>

IN RESPECT OF: <u>MiTek® Truss Designs</u>

This producer statement covers the MiTek 20/20<sup>®</sup> truss design and the structural performance of the GANG-NAIL<sup>®</sup> connector plate for the job reference **358544C1** and may be used by a Building Consent Authority to assist in determining compliance with the New Zealand Building Code.

The MiTek 20/20<sup>®</sup> truss design program has been developed by MiTek New Zealand Limited for the design of MiTek<sup>®</sup> timber roof, floor and attic trusses in New Zealand. The truss designs computed by MiTek 20/20<sup>®</sup> are prepared using sound and widely accepted engineering principles, and in accordance with compliance documents of the New Zealand Building Code and Verification Method B1/VM1; and internationally accepted standard ANSI/TPI 1 - 2002 as an alternative solution, to satisfy the requirements of Clause B1 of the New Zealand Building Code.

#### On behalf of MiTek New Zealand Limited, and subject to:

- i) All proprietary products meeting their performance specification requirements
- ii) The provision of adequate roof bracing and overall building stability
- iii) Correct selection and placement of GANG-NAIL connector plates
- iv) Correct input of Truss Design Data as shown in the Fabricator Design Statement for this job
- v) The design being undertaken by the accredited fabricator under the terms of the software licence
- vi) Timber is graded to the requirements of NZS 3603:1993
- vii) Minimum timber treatment for these MiTek<sup>®</sup> trusses shall be in accordance with B2/AS1 Table 1A and the relevant sections of NZS 3602:2003

I believe on reasonable grounds that the trusses, if constructed in accordance with the MiTek 20/20<sup>®</sup> truss design and shop drawings, will comply with the relevant provisions of the New Zealand Building Code.

MiTek New Zealand Limited holds a current policy of Professional Indemnity Insurance no less than \$500,000.

On behalf of MiTek New Zealand Limited,

Date: Tuesday, 7 December 2021

In Ling Ng, BE (Hons), CPEng, IntPE, MIPENZ (ID: 146585) TECHNICAL SERVICES MANAGER, MITek New Zealand Limited **BUILDING CONSENT** 

GRANTED 28/03/2022

HUTT CITY COUNCIL

Building Consent No.: MiTek 20/20 Engineering 4.7.346.0 Client: Phone: Fabricator Design Statement : Page

DMAC Homes Ltd T/A Friday Homes

New House- Cromwell 2 64 Waipounamu Drive (Cromwell 2) Kelson Lower Hutt

Site

Phone: Printed: 15:49:58 07 Dec 2021

#### **MITEK FABRICATOR DESIGN STATEMENT**

MiTek New Zealand Limited

This statement is issued by MiTek accredited fabricator **CARTERS**, being licensed to use the MiTek 20/20<sup>®</sup> software, to the client listed above and may be used by the Building Consent Authority to assist in determining compliance with the New Zealand Building Code.

#### MiTek 20/20<sup>®</sup> TRUSS DESIGN DATA

The MiTek 20/20° computer design for this job is based on the following design parameters entered into the program. The Fabricator shall ensure that these job details are current and relevant to the project for the design of the MiTek <sup>®</sup> trusses.

Job Details		Importance Level :	2	Design Working Life :	50 years
Roof Truss					
Timber Group:	MSG8 DDP H1.2	Pitch:	25.000 deg	Nominal Overhang:	450 mm
Roof		Ceiling		Wind	
Material:	Longrun	Material:	Gib 13mm/Rondo Screwed	Area:	High (44.0 m/s )
Dead Load:	0.210 kPa	Dead Load:	0.200 kPa	Pressure Coeff:	Cpe = varies; Cpi = -0.30, 0.20
Restraints:	900 mm centres	Restraints:	600 mm centres		
Live Load:	Qur = 0.250 kPa	Live Load:	Qc = 1.400 kN		
	Qc = 1.100 kN				

The minimum timber treatment for these MiTek<sup>®</sup> trusses shall be in accordance with B2/AS1 Table 1A and the relevant sections of NZS 3602:2003. The timber for these MiTek<sup>®</sup> trusses shall be graded to the requirements of NZS 3603:1993. Proprietary fixings and timber connectors shall be selected in accordance with NZS3604:2011 Section 4 - Durability.

#### MiTek<sup>®</sup> Truss List

\* = detail only, ? = input only, Txx = failed design, Ø = non certified, Unmarked trusses = designed successfully, LB = lateral bracing required Legend: GB = gable brace required Truss Qty Span Pitch Spacing Truss Qty Span Pitch Spacing Truss Span Pitch Spacing Qty (mm) (deg) (mm) (mm) (deg) (mm) (mm) (deg) (mm) ET1 1 3189 25.000 . 900 \*OR1 2 1305 0.000 . 900 JЗА 2 1427 25.000 . 900 \*FR1 4080 25.000 900 \*R1 2 968 25.000 900 J5A 2 1405 25.000 900 1 \*HB1 1 3771 18.249 900 \*R2 8 741 25.000 900 J5B 1 1405 25.000 900 \*HB2 2 6451 18.249 900 \*R3 2 946 25.000 900 T1 3227 25.000 900 1 \*HB3 1 1 2460 18.249 900 T1A 3227 25.000 900 T3A 2 3180 25.000 900 \*HB4 2 3637 18.249 900 T2 8 8300 25.000 900 J1B 1 2327 25.000 900 J1 2 2327 25.000 900 T3 1 3180 25.000 900 J1C 1 2327 25.000 900 J2 2327 25.000 J1D 25.000 1 3227 25.000 900 TR1 2 1 8300 900 900 1 J3 2 V1 1427 25.000 900 1474 25.000 900 TG1 8300 25.000 900 1 J4 4320 1 2327 25.000 900 J1A 2 2327 25.000 900 TG2 25.000 900 1 J5 2 1405 25.000 900 J1E 1 2327 25.000 900 TG1A 1 8300 25.000 900

Total quantity : 60

The computer design input has been carried out by:

MiTek 20/20 Software Operator:

MiTek Candidate Number and Qualifications: Ravindranath





Reshma

#### **BUILDING CONSENT**

GRANTED 28/03/2022

#### **HUTT CITY COUNCIL**

Tuesday, 7 December 2021

