GIB EzyBrace® Bracing Software

Demand Calculation Sheet

Job Details

Name:	Franz Josef
Street and Number:	Lot 55, 57, 61, 66 & 72 Stage 2 Kelson Heights
Lot and DP Number:	Lot 55, 57, 61, 66 & 72 DP TBC
City/Town/District:	Lower Hutt
Designer:	E Horner
Company:	Prime Designs
Date:	Monday, 8 November 2021

Building Specification

Number of Storeys	1
Floor Loading	2 kPa
Foundation Type	Slab
	Single
Cladding Weight	Light
Roof Weight	Light
Room in Roof Space	No
Roof Pitch (degrees)	25
Roof Height above Eaves (m)	2.8
Building Height to Apex (m)	5.3
Ground to Lower Floor (m)	0.2
Average Stud Height (m)	2.4
Building Length (m)	18.64
Building Width (m)	10.3
Building Plan Area (m²)	156.98

These documents must be retained on site. Inspections may not be carried out if they are not.

BUILDING CONSENT

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17/05/2022

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HUTT CITY TE AWA KAIRANGI

Building Location

Wind Zone = High

Earthquake Zone 3

Soil Type D & E (Deep to Very Soft)

Annual Prob. of Exceedance: 1 in 500 (Default)

Bracing Units required for Wind

	Along	Across
Single Level	**586**	**1131**

Bracing Units required for Earthquake

	Along & Across
Single Level	**834**

GIB EzyBrace® Bracing Software

Single Level Along Resistance Sheet

Job Name: Franz Josef

									Wind	EQ
									Demand	
									586	834
									Achieved	
Line	Element	Length (m)	Angle (degrees)	Stud Ht. (m)	Type	Supplier	Wind (BUs)	EQ (BUs)	1408 240%	1237 148%
a	1	2.10		2.4	GS1-N	GIB®	145	126		
	2	3.30		2.4	GS1-N	GIB®	228	198		
	External Length = 10.18								373 OK	324 OK
b	1	0.50		2.4	GS1-N	GIB®	28	29		
	2	1.09		2.4	GS1-N	GIB®	73	65		
	3	1.70		2.4	GS1-N	GIB®	117	102		
	4	2.40		2.4	GS1-N	GIB®	166	144		
	External Length = 10.46								383 OK	340 OK
c	1	1.90		2.4	GS1-N	GIB®	131	114		
	2	1.99		2.4	GS1-N	GIB®	137	119		
									268 OK	233 OK
d	1	0.90		2.4	GS1-N	GIB®	57	53		
	2	1.30		2.4	GS1-N	GIB®	90	78		
	3	1.08		2.4	GS1-N	GIB®	72	64		
	4	2.40		2.4	GS1-N	GIB®	166	144		
	External Length = 18.64								384 OK	340 OK

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GIB EzyBrace® Bracing Software

Single Level Across Resistance Sheet

Job Name: Franz Josef

									Wind	EQ
									Demand	
									1131	834
									Achieved	
Line	Element	Length (m)	Angle (degrees)	Stud Ht. (m)	Type	Supplier	Wind (BUs)	EQ (BUs)	1096 Check	1016 122%
m	1	0.74		2.4	GS1-N	GIB®	44	44		
	2	0.74		2.4	GS1-N	GIB®	44	44		
	3	0.60		2.4	GS1-N	GIB®	34	35		
	External Length = 6.36								123 OK	122 OK
n	1	2.90		2.4	GS1-N	GIB®	200	174		
	2	1.90		2.4	GS1-N	GIB®	131	114		
									331 OK	288 OK
o	1	1.45		2.4	GS1-N	GIB®	100	87		
	2	1.90		2.4	GS1-N	GIB®	131	114		
	External Length = 3.94								231 OK	201 OK
p	1	0.63		2.4	GS1-N	GIB®	36	37		
	2	2.99		2.4	GS1-N	GIB®	206	179		
									243 OK	216 OK
q	1	0.40		2.4	BL1-H	GIB®	36	40		
	2	0.40		2.4	BL1-H	GIB®	36	40		
	3	0.40		2.4	BLP-H	GIB®	48	54		
	4	0.40		2.4	BLP-H	GIB®	48	54		
	External Length = 10.3								168 OK	188 OK

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Custom Wall Elements

Supplier	System	Min. Length m	Wind BUs/m	EQ BUs/m
CHH	EP1 0.4	0.4	80	95
CHH	EP1 0.6	0.6	95	105
CHH	EP1 1.2	1.2	120	135

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BUILDABLE CONSENT LAYOUT

For valley/saddle truss fixing unless stated otherwise use a pair of wire dogs at 900mm centres for up to and including a very high wind zone. Or a pair of CT200's at 900mm centres for extra high wind zone. This fixing is to meet the minimum requirements as per NZS3604.

CARTERS Your Building Partner

BC220445

Internal Load Bearing Wall

Beams

BUILDING CONSENT

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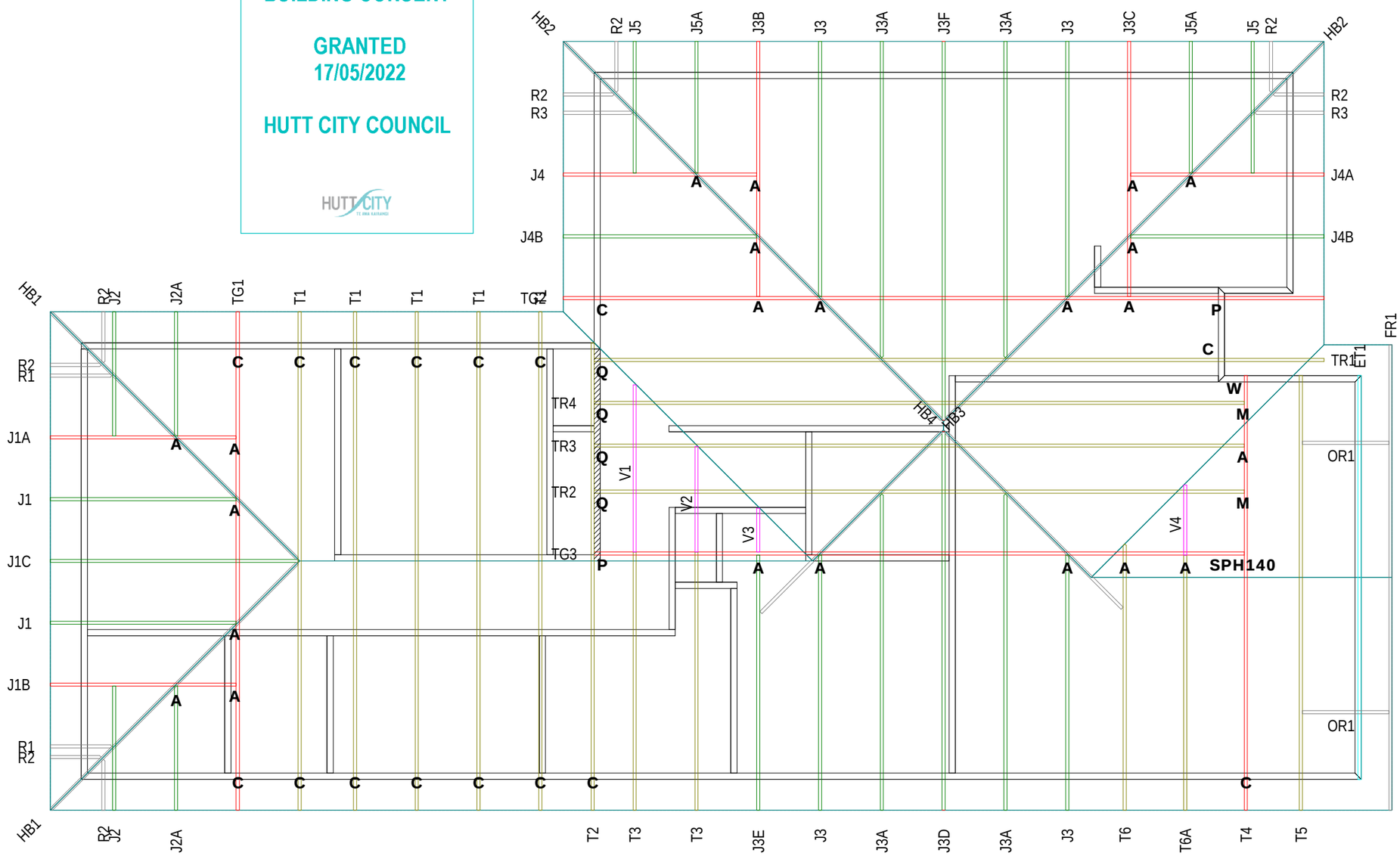

If metal ceiling battens on clips are used, 90x45 SG8 bottom chord restraints are required at 1800mm centres fixed with 2/90x3.15dia nails (skew nails if on edge).
All gable trusses are designed to suit cladding manufacturer's framing requirements.
If a gable truss requires a windbeam brace, the type of MiTek brace will be noted as such on the layout.

CARTERS
Wellington Manufacturing
(04) 5670287

JOB No **358592C1**

Client: DMAC Homes Ltd T/A Friday Homes
Job: New House-Franz Josef
Site: 64 Waipounamu Drive (Franz Josef)
Kelson
Lower Hutt

Pitch: 25.0deg
Roof Type: Longrun
Overhang: 450mm
Wind Area: High
Roof Snow: 0.000kPa
Ceiling Restraint Centres: 600mm

Trusses and rafters at 900mm max centres unless stated otherwise. This layout is to be read in conjunction with the Architectural plans.

DRAWN Reshma | 15 Dec,2021

FIXINGS

A = 47x90 Joist Hanger
B = 47x120 Joist Hanger
C = CT200 (pair)
D = 47x190 Joist Hanger
E = 95x165 Joist Hanger
F = SH-140 Split Hanger
G = SH-180 Split Hanger
H = SH-220 Split Hanger
J = 2x6kN Strap (12kN Total)
K = 6kN Strap
L = Multigrip (single)
M = Multigrips (pair)
N = Nailon Plate (240x110x1)
P = 16kN Pack
Q = 9kN Pack
S = CPC 40 Single Cleat
T = CPC 40 Short (pair)
U = CPC 80 Single Cleat
V = 16kN Uplift
W= 24kN Uplift
X = 25kN Uplift
Y = 35kN Uplift
Z = 45kN Uplift

Unless otherwise indicated, all specified truss fixings are to use L/Lok product nail fasteners (as per the MiTek On-site Guide) when the choice of using screws or nails is optional.

All truss to frame fixings require 2 additional 2/90x3.15dia skew nails.

All truss fixings not indicated as above must have 2 wire dogs for cross joints and 2/90x3.15dia nails for butt joins.

Fixings shown are for fixing trusses to the top plate. Any other point load uplift fixings down through the framing stud to top plate, stud to bottom plate, bottom plate to floor remain the responsibility of the architect / draughtsman.

Truss Layout

BUILDABLE CONSENT LAYOUT

BC220445

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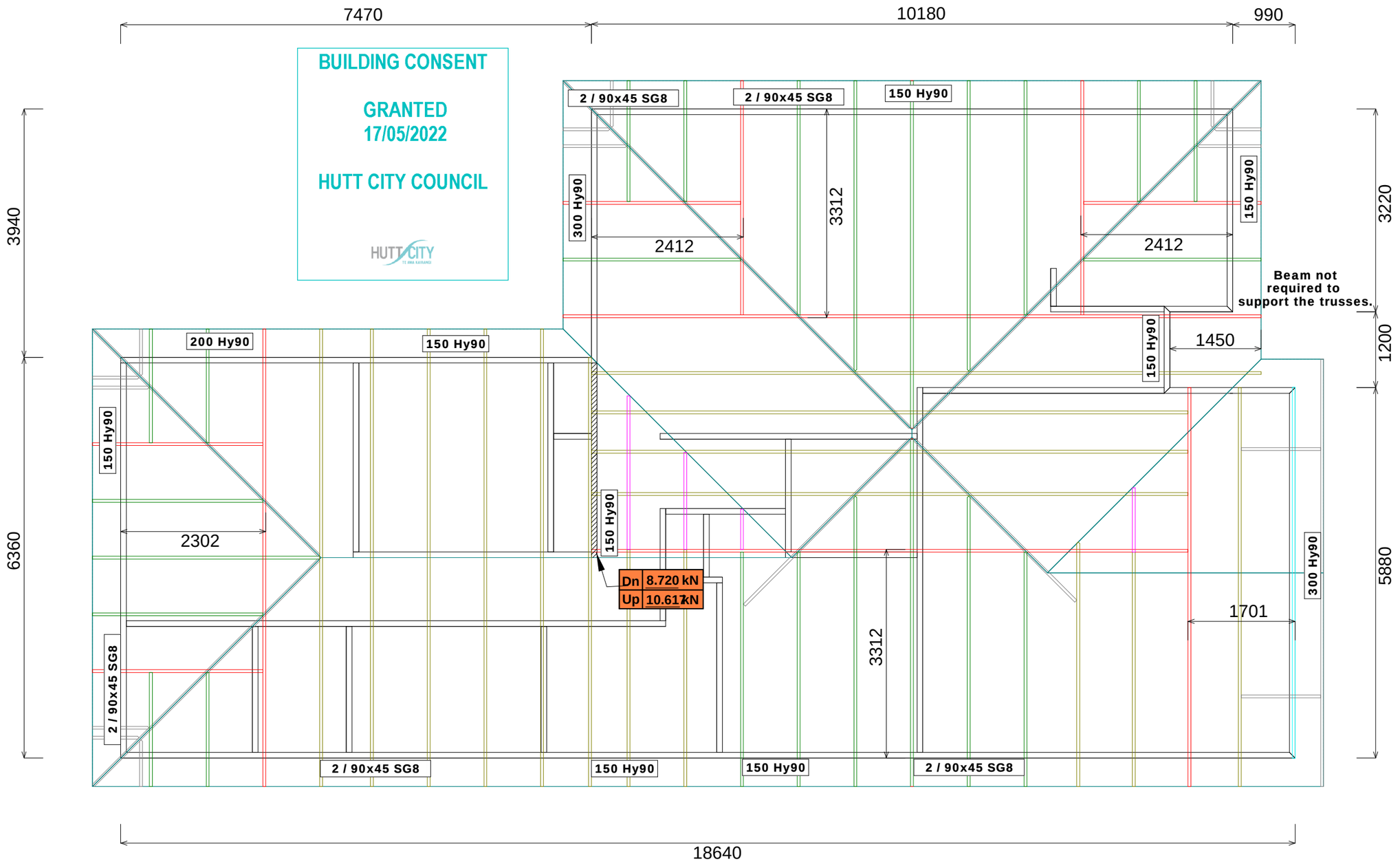
Internal Load Bearing Wall

Beams



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Wellington Manufacturing
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JOB No **358592C1**

Client: DMAC Homes Ltd T/A Friday Homes
Job: New House-Franz Josef
Site: 64 Waipounamu Drive (Franz Josef)
Kelson
Lower Hutt

Pitch: 25.0deg
Roof Type: Longrun
Overhang: 450mm
Wind Area: High
Roof Snow: 0.000kPa

Trusses and rafters at 900 mm max centres unless stated otherwise. This layout is to be read in conjunction with the Architectural plans.

DRAWN Reshma | 15 Dec,2021

Dn kN | Up kN — Ultimate Limit State Loads

Notification of point loaded lintels or point loads on internal walls where the downward load is higher than 8kN (85mm raft type slab) or 10kN (100mm standard slab), or the upward load is greater than 10kN.

Any roof loads as stated on this layout over 16kN up or down are outside the scope of NZS3604, and the architect / draughtsperson is responsible for the design to transfer the loads to the ground.

The lintels have been sized using one of the following:

hy90, hyONE and hySPAN lintels have been sized using the designIT for houses - New Zealand series 6 software.

GANGLAM and FLITCH BEAMS have been sized using the MiTek Beam Program V1.10 June 2011.

Unless otherwise stated the timber grade for all lintels is SG8. Lintels not shown are to be selected as per NZS3604: 2011.

All internal walls shown hatched on this layout are considered to be loadbearing
Lintel fixing specification remains the responsibility of the architect / draughtsperson

MiTek New Zealand Limited

Correspondence from : **AUCKLAND**
40 Neales Road, East Tamaki 2013
PO Box 58-014, Botany 2163
Phone: 09 274 7109
Fax: 09 274 7100

CHRISTCHURCH
14 Pilkington Way, Wigram 8042
PO Box 8387, Riccarton 8440
Phone: 03 348 8691
Fax: 03 348 0314

www.miteknz.co.nz

PRODUCER STATEMENT for MiTek 20/20® TRUSS DESIGN - Version 4.7

ISSUED BY: **MiTek New Zealand Limited**

TO: **CARTERS**

IN RESPECT OF: **MiTek® Truss Designs**

This producer statement covers the MiTek 20/20® truss design and the structural performance of the GANG-NAIL® connector plate for the job reference **358592C1** and may be used by a Building Consent Authority to assist in determining compliance with the New Zealand Building Code.

The MiTek 20/20® truss design program has been developed by MiTek New Zealand Limited for the design of MiTek® timber roof, floor and attic trusses in New Zealand. The truss designs computed by MiTek 20/20 ® are prepared using sound and widely accepted engineering principles, and in accordance with compliance documents of the New Zealand Building Code and Verification Method B1/VM1; and internationally accepted standard ANSI/TPI 1 - 2002 as an alternative solution, to satisfy the requirements of Clause B1 of the New Zealand Building Code.

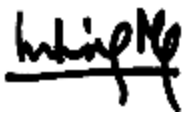
On behalf of MiTek New Zealand Limited, and subject to:

i) All proprietary products meeting their performance specification requirements
ii) The provision of adequate roof bracing and overall building stability
iii) Correct selection and placement of GANG-NAIL connector plates
iv) Correct input of Truss Design Data as shown in the Fabricator Design Statement for this job
v) The design being undertaken by the accredited fabricator under the terms of the software licence
vi) Timber is graded to the requirements of NZS 3603:1993
vii) Minimum timber treatment for these MiTek® trusses shall be in accordance with B2/AS1 Table 1A and the relevant sections of NZS 3602:2003

I believe on reasonable grounds that the trusses, if constructed in accordance with the MiTek 20/20® truss design and shop drawings, will comply with the relevant provisions of the New Zealand Building Code.

MiTek New Zealand Limited holds a current policy of Professional Indemnity Insurance no less than $500,000.

On behalf of MiTek New Zealand Limited, **Date: Wednesday, 15 December 2021**



In Ling Ng, BE (Hons), CPEng, IntPE, MIPENZ (ID: 146585)
TECHNICAL SERVICES MANAGER, MiTek New Zealand Limited

BC220445

Job: 358592C1 Client: DMAC Homes Ltd T/A Friday Homes Site: New House-Franz Josef
Phone: 64 Waipounamu Drive (Franz Josef)
Description: Kelson
Building Consent No.: Lower Hutt Phone:

MITEK FABRICATOR DESIGN STATEMENT

This statement is issued by MiTek accredited fabricator **CARTERS**, being licensed to use the MiTek 20/20® software, to the client listed above and may be used by the Building Consent Authority to assist in determining compliance with the New Zealand Building Code.

MiTek 20/20® TRUSS DESIGN DATA

The MiTek 20/20® computer design for this job is based on the following design parameters entered into the program. The Fabricator shall ensure that these job details are current and relevant to the project for the design of the MiTek ® trusses.

Job Details

		Importance Level :	2	Design Working Life :	50 years
Roof Truss					
Timber Group:	MSG8 DDP H1.2	Pitch:	25.000 deg	Nominal Overhang:	450 mm
Roof		**Ceiling**		**Wind**	
Material:	Longrun	Material:	Gib 13mm/Rondo Screwed	Area:	High (44.0 m/s)
Dead Load:	0.210 kPa	Dead Load:	0.200 kPa	Pressure Coeff:	Cpe = varies; Cpi = -0.30, 0.20
Restraints:	900 mm centres	Restraints:	600 mm centres		
Live Load:	Qur = 0.250 kPa Qc = 1.100 kN	Live Load:	Qc = 1.400 kN		

The minimum timber treatment for these MiTek® trusses shall be in accordance with B2/AS1 Table 1A and the relevant sections of NZS 3602:2003. The timber for these MiTek ® trusses shall be graded to the requirements of NZS 3603:1993. Proprietary fixings and timber connectors shall be selected in accordance with NZS3604:2011 Section 4 - Durability.

MiTek® Truss List

Legend: * = detail only, ? = input only, ~~Txx~~ = failed design, Ø = non certified, Unmarked trusses = designed successfully, LB = lateral bracing required
GB = gable brace required

Truss	Qty	Span (mm)	Pitch (deg)	Spacing (mm)	Truss	Qty	Span (mm)	Pitch (deg)	Spacing (mm)	Truss	Qty	Span (mm)	Pitch (deg)	Spacing (mm)	
ET1	1	5889	25.000	900	J3D	1	3267	25.000	900	T5	1	5880	25.000	900	
*FR1	1	6780	25.000	900	J3E	1	3267	25.000	900	T6	1	3267	25.000	882	
*HB1	2	5079	18.249	900	J3F	1	3267	25.000	900	T6A	1	3267	25.000	882	
*HB2	2	7780	18.249	900	J4	1	2367	25.000	900	TG1	1	6360	25.000	900	
*HB3	1	3733	18.249	900	J4A	1	2367	25.000	900	TG2	1	10180	25.000	900	
*HB4	1	3644	18.249	900	J4B	2	2367	25.000	900	TG3	1	9469	25.000	900	
J1	2	2257	25.000	900	J5	2	1467	25.000	900	TR1	1	10180	25.000	900	
J1A	1	2257	25.000	900	J5A	2	1467	25.000	900	TR2	1	9469	25.000	900	LB
J1B	1	2257	25.000	900	*OR1	2	1260	0.000	900	TR3	1	9469	25.000	625	
J1C	1	2257	25.000	900	*R1	2	898	25.000	900	TR4	1	9469	25.000	900	
J2	2	1357	25.000	900	*R2	8	741	25.000	900	V1	1	2436	25.000	900	
J2A	2	1357	25.000	900	*R3	2	1008	25.000	900	V2	1	1536	25.000	900	
J3	4	3267	25.000	900	T1	5	6360	25.000	900	V3	1	636	25.000	900	
J3A	4	3267	25.000	900	T2	1	6360	25.000	900	V4	1	1020	25.000	900	
J3B	1	3267	25.000	900	T3	2	3312	25.000	900						
J3C	1	3267	25.000	900	T4	1	5880	25.000	900						

Total quantity : 75

The computer design input has been carried out by:

MiTek 20/20 Software Operator: Reshma

MiTek Candidate Number and Qualifications: **Ravindranath**

Accredited Fabricator:

CARTERS
Your Building Partner

BUILDING CONSENT

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17/05/2022

HUTT CITY COUNCIL

Wednesday, 15 December 2021

HUTT CITY
TE AWA KAIRANGI

designIT® HOUSES

CarterHoltHarvey LVL

DESIGN CERTIFICATE

Technical basis for structural design methodology contained in designIT for houses - New Zealand.

designIT for houses, New Zealand has been developed by experienced timber engineers to assist designers in selecting appropriate sizes of structural laminated veneer lumber products manufactured by Carter Holt Harvey LVL Limited (including hySPAN, hy90, hyONE and hyJOIST) and other generic stress grades of timber, to be used as structural elements for the construction of buildings that fall within the scope of NZS 3604.

The design methodology used for the software complies with the loading and general design requirements contained within AS/NZS 1170 and with timber structural design in accordance with NZS 3603:1993 including Amendment 4 (Verification method B1/VM1, 6.1).

designIT relies on the accurate input of span and loading information by the user. Where accurate inputs are submitted the product and/or stress grade and the size given will comply with the structural requirements of the New Zealand Building Code (NZBC), provided the installation is in accordance with the installation requirements provided by designIT and/or in product literature and/or NZS 3604, or specific engineering design, as appropriate.

Futurebuild LVL and SG8 components, when used and treated to the required treatment levels prescribed in NZS 3602 and NZS 3604, as modified by Acceptable Solution B2/AS1, will comply with the requirements of the NZBC (Acceptable Solution B2/AS1, 3.2).

References:
1. NZS 3603:1993 Timber Structures Standard.
2. NZS 3604:2011 Timber-framed buildings.
3. AS/NZS 1170:2002 Structural design actions, Parts 0 and 1.
4. AS/NZS 1170:2011 Structural design actions, Part 2: Wind actions.
5. AS/NZS 1170:2003 Structural design actions, Part 3: Snow and ice actions.
6. AS 1720.1:2010 Timber structures. Part 1: Design methods.
7. AS 1720.3:2016 Timber structures. Part 3: Design criteria for timber-framed residential buildings.

This Design Certificate, and any associated warranty/certification, is void where there has been substitution of alternate products not detailed within the Member Specification.

Version date: 5 October 2021

For further information or advice contact:
Carter Holt Harvey LVL Limited,
173 Captain Springs Road, Onehunga. Auckland
Telephone: 0800 808 131
Email: designit@futurebuild.co.nz
Web: https://futurebuild.co.nz/

Specifier details:

Specifier:	Reshma Ravindranath
Business name:	Carters Manufacturing Wellington
Address:	Lower Hutt
Email:	reshma.ravindranath@carters.co.nz

Project & site details:

Project:	New House-Franz Josef
Reference:	358592C1
Site address:	64 Waipounamu Drive Kelson Lower Hutt
For (owner/s):	D.M.A.C Homes Limited | T/A Friday Homes
Design wind zone	High
Snow loading	Design snow zone: N1, Altitude: 100 m (sub-alpine), Ground snow load, $S_g^{1,2}$ = 0.0 kPa

. designIT does not include any allowance for the effects of drifting and sliding of snow.
. Snow loads are applied to roofed over structures only, the design of exposed floors/decks are not covered by designIT.

MEMBER DESIGN DETAILS

Member 1

1) Member code and description EJ01 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	1.0 m
Roof load width 'RLW'	7.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - $G + \Psi_{LQ}$*	3.3 mm	0.6 mm (long term)	5.7
Live load - $\Psi_S Q$	4.0 mm	0.2 mm	22.0
Wind load - W_S	5.0 mm	0.5 mm	9.1

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-3.7
1.2G + 1.5Q	0.80	-5.9
1.2G + W_u + $\Psi_c Q$	1.00	-8.3
0.9G + W_u	1.00	5.4

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 2

1) Member code and description EJ02 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	2.3 m
Roof load width 'RLW'	1.9 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_{LQ}*	7.7 mm	2.1 mm (long term)	3.7
Live load - Ψ_{s}Q	9.2 mm	0.6 mm	16.0
Wind load - W_s	11.5 mm	1.7 mm	6.6

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.7
1.2G + 1.5Q	0.80	-2.6
1.2G + W_u + Ψ_cQ	1.00	-3.7
0.9G + W_u	1.00	2.3

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 3

1) Member code and description EJ03 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	2.3 m
Roof load width 'RLW'	2.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_{LQ}*	7.7 mm	2.5 mm (long term)	3.1
Live load - Ψ_{s}Q	9.2 mm	0.7 mm	13.2
Wind load - W_s	11.5 mm	2.1 mm	5.5

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-2.0
1.2G + 1.5Q	0.80	-3.1

1.2G + W_u + Ψ_cQ	1.00	-4.4
0.9G + W_u	1.00	2.8

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 4

1) Member code and description EJ04 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	0.7 m
Roof load width 'RLW'	2.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_LQ*	2.3 mm	0.4 mm (long term)	6.6
Live load - Ψ_sQ	2.8 mm	0.1 mm	25.8
Wind load - W_s	3.5 mm	0.3 mm	10.7

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.2
1.2G + 1.5Q	0.80	-1.9
1.2G + W_u + Ψ_cQ	1.00	-2.7
0.9G + W_u	1.00	1.8

8) Installation requirements

- Provide at least 30 mm bearing at end supports
- Vertical lamination required - refer AS 1684

BUILDING CONSENT GRANTED 17/05/2022 HUTT CITY COUNCIL

HUTT CITY TE AWA KAIRANGI

Member 5

1) Member code and description EJ05 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	0.7 m
Roof load width 'RLW'	1.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_{LQ}*	2.3 mm	0.2 mm (long term)	11.6
Live load - $\Psi_{S}Q$	2.8 mm	0.1 mm	45.6
Wind load - W_S	3.5 mm	0.2 mm	18.9

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

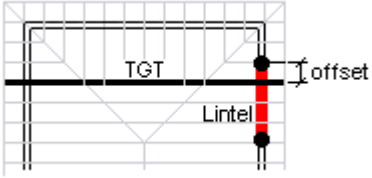
Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-0.7
1.2G + 1.5Q	0.80	-1.1
1.2G + W_u + Ψ_cQ	1.00	-1.5
0.9G + W_u	1.00	1.0

8) Installation requirements

- Provide at least 30 mm bearing at end supports
- Vertical lamination required - refer AS 1684

Member 6

1) Member code and description EJ06 - Lintels - Supporting girder truss / truncated girder truss



2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Roof type	Light roof and ceiling - 40 kg/m²
Roof pitch	25.0 º

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Truss spacing	900 mm
Eaves overhang	600 mm
TGT span	10180 mm
TGT setback	3267 mm
TGT offset	3117 mm
Lintel span	3320 mm
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 300 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - $G + \Psi_L Q^*$	10.0 mm	2.5 mm (long term)	4.0
Live load - $\Psi_s Q$	13.3 mm	0.7 mm	19.0
Wind load - W_s	16.6 mm	2.1 mm	7.9

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-7.6
1.2G + 1.5Q	0.80	-11.8
$1.2G + W_u + \Psi_c Q$	1.00	-16.7
$0.9G + W_u$	1.00	10.6

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 7

1) Member code and description	EJ07 - Lintels - In single or upper storey load bearing walls
2) Date prepared	15 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	2.1 m
Roof load width 'RLW'	4.1 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

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Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_{LQ}*	7.0 mm	3.2 mm (long term)	2.2
Live load - $\Psi_{S}Q$	8.4 mm	0.9 mm	8.9
Wind load - W_S	10.5 mm	2.9 mm	3.7

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

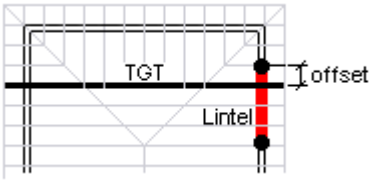
Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-3.3
1.2G + 1.5Q	0.80	-5.2
1.2G + W_u + Ψ_cQ	1.00	-7.3
0.9G + W_u	1.00	4.7

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 8

1) Member code and description EJ08 - Lintels - Supporting girder truss / truncated girder truss



2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Roof type	Light roof and ceiling - 40 kg/m²
Roof pitch	25.0 º
Truss spacing	900 mm
Eaves overhang	600 mm
TGT span	6360 mm
TGT setback	2257 mm
TGT offset	1567 mm
Lintel span	2420 mm
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 200 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

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6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_{LQ}*	8.1 mm	2.9 mm (long term)	2.7

Live load - $\Psi_s Q$	9.7 mm	0.9 mm	11.2
Wind load - W_s	12.1 mm	2.6 mm	4.6

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-2.5
1.2G + 1.5Q	0.80	-3.9
1.2G + W_u + $\Psi_c Q$	1.00	-5.4
0.9G + W_u	1.00	3.4

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 9

1) Member code and description EJ09 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	2.5 m
Roof load width 'RLW'	1.8 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + $\Psi_L Q$*	8.3 mm	2.8 mm (long term)	2.9
Live load - $\Psi_s Q$	10.0 mm	0.8 mm	12.8
Wind load - W_s	12.5 mm	2.4 mm	5.3

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.7
1.2G + 1.5Q	0.80	-2.6
1.2G + W_u + $\Psi_c Q$	1.00	-3.6
0.9G + W_u	1.00	2.2

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 10

1) Member code and description EJ10 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	0.7 m
Roof load width 'RLW'	1.8 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - $G + \Psi_{LQ}Q^*$	2.3 mm	0.3 mm (long term)	8.4
Live load - $\Psi_S Q$	2.8 mm	0.1 mm	33.0
Wind load - W_S	3.5 mm	0.3 mm	13.6

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.0
1.2G + 1.5Q	0.80	-1.5
$1.2G + W_u + \Psi_c Q$	1.00	-2.1
$0.9G + W_u$	1.00	1.4

8) Installation requirements

- Provide at least 30 mm bearing at end supports
- Vertical lamination required - refer AS 1684

Member 11

1) Member code and description EJ11 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	0.7 m
Roof load width 'RLW'	4.1 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + Ψ_{LQ}*	2.3 mm	0.6 mm (long term)	3.7
Live load - $\Psi_{s}Q$	2.8 mm	0.2 mm	14.5
Wind load - W_s	3.5 mm	0.6 mm	6.0

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-2.2
1.2G + 1.5Q	0.80	-3.4
1.2G + W_u + $\Psi_c Q$	1.00	-4.8
0.9G + W_u	1.00	3.2

8) Installation requirements

- Provide at least 30 mm bearing at end supports
- Vertical lamination required - refer AS 1684

Member 12

1) Member code and description	EJ12 - Lintels - In single or upper storey load bearing walls
2) Date prepared	15 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	1.7 m
Roof load width 'RLW'	2.4 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]

Long term load - G + $\Psi_L Q^*$	5.7 mm	1.0 mm (long term)	5.6
Live load - $\Psi_s Q$	6.8 mm	0.3 mm	23.2
Wind load - W_s	8.5 mm	0.9 mm	9.6

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.7
1.2G + 1.5Q	0.80	-2.6
1.2G + W_u + $\Psi_c Q$	1.00	-3.7
0.9G + W_u	1.00	2.3

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 13

1) Member code and description EJ13 - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	2.1 m
Roof load width 'RLW'	2.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + $\Psi_L Q^*$	7.0 mm	1.9 mm (long term)	3.8
Live load - $\Psi_s Q$	8.4 mm	0.5 mm	15.8
Wind load - W_s	10.5 mm	1.6 mm	6.6

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.9
1.2G + 1.5Q	0.80	-3.0
1.2G + W_u + $\Psi_c Q$	1.00	-4.2
0.9G + W_u	1.00	2.6

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 14

1) Member code and description	EJ14 - Lintels - In single or upper storey load bearing walls
2) Date prepared	15 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016
4) Design inputs	
Span	0.9 m
Roof load width 'RLW'	2.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm
5) Member specification	
Size, stress grade/product	Use 2/90 x 45 SG8
Material type	Dry softwood, machine stress graded and verified (NZS 3622)
Assumed design density	< 480 kg/m²

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - $G + \Psi_L Q^*$	3.0 mm	0.8 mm (long term)	4.0
Live load - $\Psi_S Q$	3.6 mm	0.2 mm	15.6
Wind load - W_S	4.5 mm	0.7 mm	6.5

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-1.3
1.2G + 1.5Q	0.80	-2.0
$1.2G + W_u + \Psi_c Q$	1.00	-2.9
$0.9G + W_u$	1.00	1.9

8) Installation requirements

- Provide at least 30 mm bearing at end supports
- Vertical lamination required - refer AS 1684

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Member 15

1) Member code and description	GD01 - Lintels - In single or upper storey load bearing walls
2) Date prepared	15 December 2021
3) Serviceability criteria	AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	4.9 m
Roof load width 'RLW'	1.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 300 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]
Long term load - G + $\Psi_L Q^*$	10.0 mm	5.2 mm (long term)	1.9
Live load - $\Psi_S Q$	15.0 mm	1.2 mm	12.1
Wind load - W_S	24.5 mm	3.7 mm	6.6

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-2.3
1.2G + 1.5Q	0.80	-3.4
1.2G + W_u + $\Psi_c Q$	1.00	-4.6
0.9G + W_u	1.00	2.4

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Member 16

1) Member code and description HWC Internal - Lintels - In single or upper storey load bearing walls

2) Date prepared 15 December 2021

3) Serviceability criteria AS 1720.1: 2010 and AS 1720.3: 2016

4) Design inputs

Span	1.6 m
Roof load width 'RLW'	5.3 m
Roof type and mass	Light roof and ceiling - 40 kg/m²
Nominal wall thickness	90 mm

5) Member specification

Size, stress grade/product	Use 150 x 90 hy90
Material type	Structural Laminated Veneer Lumber to AS/NZS 4357

6) Serviceability

Load case	Limit[3] on average deflection[2]	Estimated average deflection[2]	Rigidity ratio[4]

Long term load - G + $\Psi_L Q^*$	5.3 mm	1.8 mm (long term)	3.0
Live load - $\Psi_S Q$	6.4 mm	0.5 mm	11.8
Wind load - W_S	8.0 mm	1.6 mm	4.9

*Critical serviceability load case
See 'Notes for interpretation of serviceability data' at the end of this report

7) Reactions

Load case	k_1[1]	Limit States Design Reaction[2,3] End kN[4]
1.35G	0.60	-3.4
1.2G + 1.5Q	0.80	-5.3
1.2G + W_u + $\Psi_c Q$	1.00	-7.5
0.9G + W_u	1.00	4.9

8) Installation requirements

- Provide at least 30 mm bearing at end supports

Notes for interpretation of serviceability data

1. 'average deflection' is an engineering concept based upon a notional estimated load, notional member rigidity and, in some cases, an approximate model of material response to environmental conditions. These parameters are, 'standardised' in AS 1170 and AS 1720.

2. Deflection is the flexural response to load 'out-of-level' measurements of installations are not necessarily deflections and can incorporate 'initial out-of-straightness', whether intended or not. Furthermore, loads can be higher/lower than the notional estimate and in any comparison with measured levels, material variability needs to also be considered. AS 1720 gives the following basis for estimation of upper bound deflections for various materials.

No 1 Framing – visually graded to NZS 3631	Average + 100%
SG grades - mechanically graded to AS/NZS 1748	Average + 43%
GL grades for glulam to AS 1328	Average + 33%
LVL to AS/NZS 4357 (includes hySPAN and hyJOIST)	Average +18%

As can be seen, comparison of the 'average deflection' for different materials, even if calculated on the same basis, does not give the whole picture!

3. The limits referred are those specified in AS 1720.3 for the stated load case.

4. 'Rigidity ratio' expresses the rigidity of the specified beam relative to the rigidity of a notional beam just meeting the serviceability requirements detailed.

Notes for interpretation of reaction data

. Duration of load factor 'k_1' for strength as per NZS 3603:1993
. Negative (-) reactions relate to the 'gravity' or 'downwards' force on the support
. Positive reactions relate to the 'upwards' forces or 'tie-down' requirement on the support
. End reaction includes allowance for overhang/cantilever where one has been designed